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ADDENDUM

Client: Palmyra United Methodist Church

Project Name: New Building for Palmyra United Methodist Church

Project Number: 5356

Addendum Number: 03

Issued: 06-12-2019

This addendum becomes a part of the bidding and contract documents and modifies the drawings and specifications dated May 17, 2019. Acknowledge receipt of this addendum by noting such on the Contractor's Proposal (Bid) Form.

FAILURE TO DO SO MAY SUBJECT BIDDER TO DISQUALIFICATION

This addenda and all future addendums with a Plan Holders List will be also be posted on the website of Architechnics, Inc. and updated daily. Check the Current Projects tab on the site: www.architechnicsinc.com

ITEM	DESCRIPTION	NOTES
DRAWINGS:		
S002	Revise	Revise Code Reference to IBC 2012, Replace with attached sheet.

This addendum consists of 2 pages;Drawings S002

TESTING AND INSPECTIONS

1. ALL TESTING SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF THE DESIGN CODE REFERENCED IN ITEM 1. OF THE STRUCTURAL LOADING SECTION OF THESE NOTES.

2. ALL TESTING SHALL BE PERFORMED BY A QUALIFIED TESTING AGENCY HIRED BY THE OWNER.

3. THE ARCHITECT / STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ANY ITEM FOUND NOT TO BE IN COMPLIANCE WITH THE DESIGN INTENT OF THESE DOCUMENTS.

FOUNDATIONS

4. ALL FOUNDATION EXCAVATIONS SHALL BE OBSERVED AND TESTED BY A REPRESENTATIVE OF A QUALIFIED GEOTECHNICAL ENGINEERING FIRM. DAILY REPORTS OF OBSERVATIONS SHALL BE PREPARED. ALL REPORTS ARE TO BE SUBMITTED TO THE ARCHITECT / STRUCTURAL ENGINEER FOR REVIEW. THE REQUIRED TEST TYPE AND FREQUENCY SHALL BE AS SPECIFIED IN THE PROJECT SPECIFICATIONS.

CONCRETE

5. ALL CONCRETE PLACED ON THE PROJECT SHALL BE TESTED FOR SLUMP, AIR CONTENT AND STRENGTH. THE FREQUENCY OF TESTING SHALL BE AS SPECIFIED IN THE PROJECT SPECIFICATION.

6. REINFORCEMENT PLACEMENT SHALL BE INSPECTED BY THE OWNER'S TESTING LABORATORY PRIOR TO ALL CONCRETE POURS. SEE THE SPECIFICATIONS FOR REQUIREMENTS.

STRUCTURAL STEEL

7. THE OWNER'S TESTING AGENCY SHALL PERFORM ALL SHOP AND FIELD INSPECTION AND TESTING AS OUTLINED BELOW. REPORTS ARE TO BE SUBMITTED TO THE OWNER, ARCHITECT / STRUCTURAL ENGINEER AND CONTRACTOR FOR REVIEW. THE STRUCTURAL STEEL FABRICATOR AND ERECTOR SHALL SCHEDULE ALL WORK TO ALLOW THE FOLLOWING TESTING REQUIREMENTS.

A. ALL WELDS SHALL BE VISUALLY INSPECTED. 15% AT RANDOM SHALL BE MEASURED.
B. FILLET WELDS FOR BEAM AND GIRDER SHEAR CONNECTION PLATES, 15% AT RANDOM, SHALL BE CHECKED BY MAGNETIC PARTICLE FOR FINAL PASS ONLY.
C. 100% OF ALL FULL PENETRATION WELDS SHALL BE ULTRASONICALLY TESTED.
D. 25% OF THE BOLTS, NO LESS THAN (2) BOLTS, IN EACH "SLIP CRITICAL" CONNECTIONS SHALL BE CHECKED BY CALIBRATED TORQUE WRENCH.
E. FOR NON-"SLIP CRITICAL" CONNECTIONS, INSPECT CONNECTION TO INSURE THE PLIES OF THE CONNECTED ELEMENTS HAVE BEEN BROUGHT INTO SNUG CONTACT.
F. ULTRASONICALLY TEST FOR LAMINATIONS IN ALL COLUMN FLANGES GREATER THAN 1.5 INCHES THICK AT ALL MOMENTS CONNECTION AREAS.

8. REFER TO PROJECT SPECIFICATIONS FOR ADDITIONAL TESTING REQUIREMENTS.

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PALMYRA UNITED METHODIST CHURCH DESIGN CRITERIA

1. BUILDING CODES:
   A. IBC 2012
   C. ASCE 7-10
2. DESIGN LOADS:
   A. OCCUPANCY CATEGORY III
   B. DEAD LOADS:
      1. STANDING SEAM METAL ROOF AT METAL BUILDING SYSTEM
         a. STANDING SEAM ROOFING = 3 PSF
         b. INSULATION = 2 PSF
         c. PURLIN FRAMING = 2 PSF
         d. MECHANICAL, ELECTRICAL & PLUMBING = 5 PSF
         e. SPRINKLER SYSTEM = 4 PSF (1)
         f. CEILING = 2 PSF

      2. WOOD JOIST FRAMING WITH PLYWOOD SHEATHING
         a. STANDING SEAM ROOFING = 3 PSF
         b. INSULATION = 2 PSF
         c. PLYWOOD SHEATHING = 2 PSF
         c. ENGINEERED WOOD FRAMING = 3 PSF
         d. MECHANICAL, ELECTRICAL & PLUMBING = 5 PSF
         e. SPRINKLER SYSTEM = 4 PSF (1)
         f. CEILING = 2 PSF

   C. ROOF LIVE LOAD = 20 PSF (TABLE 4-1)
   D. ROOF SNOW LOADS:
      1. GROUND SNOW LOAD Pg = 20 PSF
      2. THERMAL FACTOR Ct = 1.0
      3. EXPOSURE FACTOR Ce = 1.0
      3. IMPORTANCE FACTOR Is = 1.1
      4. FLAT ROOF SNOW LOAD Pf = 22 PSF (MINIMUM)
      5. RAIN-ON-SNOW SURCHARGE = 5 PSF (3)
      6. DRIFTING AND SLIDING LOADS - PER ASCE 7-10.
   E. PONDING
      1. PONDING IS NOT APPLICABLE FOR ROOF SLOPES 1/4" OR GREATER
   F. WIND LOADING - ANALYTICAL PROCEDURE
      1. BASIC WIND SPEED (3 SECOND GUST) = 120 MPH
      2. EXPOSURE CATEGORY C
      3.
      4. DIRECTIONAL FACTOR Kd = 0.85
      5. TOPOGRAPHIC FACTOR Kzt = 1.0
      6. INTERNAL PRESSURE COEFFICIENT GCPi = +/- 0.18 (ENCLOSED) (3)
      7. MAIN WIND FORCE RESISTING SYSTEM PRESSURES:
         a. PER ASCE 7-10 MAIN FORCE RESISTING SYSTEM: CHAPTER 27.
      8. DESIGN PRESSURES FOR COMPONENTS AND CLADDING:
         a. PER ASCE 7-10 COMPONENTS AND CLADDING: CHAPTER 30: PART 2.
   G. SEISMIC LOADING - EQUIVALENT LATERAL FORCE PROCEDURE:
      1. IMPORTANCE FACTOR Ie = 1.25
      2. SITE CLASS C
      3. Sds = 0.120 (Ss = 0.150) (3)
      4. Sd1 = 0.098 (S1 = 0.087)
      5. SEISMIC DESIGN CATEGORY B
      6. DESIGN COEFFICIENTS AND FACTORS FOR SEISMIC FORCE-RESISTING SYSTEMS
         a. ASCE 7-105 - TABLE 12.2-1
            i. RESISTING SYSTEM - STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.
            ii. RESPONSE COEFFICIENT, R = 3.0
            iii. DEFLECTION AMPLIFICATION FACTOR Cd = 3.0
            iv. SYSTEM OVERSTRENGTH FACTOR Xo = 3.0
      7. COMPONENT DESIGN PER ASCE 7-10 (3)

SCHEDULE OF BUILDING DESIGN LOADS

LOCATION	FLOOR	FLOOR AREA	FLOOR / ROOF CONSTRUCTION	SUPERIMPOSED DEAD LOAD (psf)	PART'N LOAD (psf)	LIVE LOAD (psf)	REMARKS
MAIN BUILDING	1ST	LOBBY	5" SLAB-ON-GRADE	15	-	100	
		PUBLIC AREAS & CORRIDORS	"	15	-	100	
		OFFICE	"	15	20	50	
		STORAGE	"	15	-	125	
		STAIRS / LADDERS	"	-	-	100	
		STAGE	"	15	-	100	
		CLASSROOM	"	15	-	40	
		MECHANICAL	"	15	-	125	MECHANICAL UNIT WEIGHTS
	ROOF	TYPICAL	*	**	-	22	SNOW DRIFT
	CANOPY ROOF	ENTRANCE CANOPY	OPEN WEB TRUSSES / PLYWOOD SHEATHING	5	-	22	SNOW DRIFT

NOTES:
1. DURING CONSTRUCTION ALL CONSTRUCTION LOADS ON ANY AREA OF THE FLOOR SHALL NOT EXCEED THE LOADS SHOWN IN THE TABLE.
2. 5" SLAB-ON-GRADE = 63 PSF
3. SUPERIMPOSED DEAD LOADS NOTED ABOVE DO NOT INCLUDE SELF WEIGHT OF STEEL JOIST FRAMING.
4. * - INDICATES EITHER:
   1. STEEL PURLINS ON PRE-ENGINEERED METAL BUILDING SYSTEM - RIGID METAL FRAME OR
   2. ENGINEERED WOOD JOISTS WITH PLYWOOD SHEATHING. (2)
5. ** - SEE BUILDIGN DESIGN LOADS FOR APPLICABLE DEAD LOADS.
6. LOADS APPLIED TO PRE-ENGINEERED METAL BUILDING FROM TIMBER FRAMING: PDL = 3.4 KIPS, PSL = 9.2 KIPS.
7. METAL BUILDING SUPPLIER AND G.C. TO COORDINATE SUPPLEMENTAL FRAMING REQUIRED AND LOADS FOR STEEPLE (ALTERNATE)

BUILDING DEFLECTION LIMITS

LOCATION	LIMITS	LOAD CASE / COMBINATION	RAFTERS L /	PURLINS L /	RAFTERS L /	REMARKS
MAIN BUILDING	ROOF LIMITS					
		LIVE	240	240	60	
		SNOW	240	240	60	
		WIND (SERVICE)	240	240	60	
		TOTAL GRAVITY	240	240	60	
		TOTAL UPLIFT	NA	240	60	
	FRAME LIMITS		SIDESWAY H /	PORTAL FRAME SIDEWAY - H /		
		LIVE	180			
		SNOW	180			
		WIND (SERVICE)	180			
		SEISMIC DRIFT	40	40		
		SERIVCE-LEVEL CRANE	NA	NA		
		PORTAL WIND (SERVICE)	NA	60		
		TOTAL GRAVITY	180			
		SERVICE SEISMIC	180	50		
	WALL LIMITS		LIMIT L /			
		TOTAL WIND (PANELS)	60			
		TOTAL WIND (GIRTS)	90			
		TOTAL WIND (COLUMNS)	180			

NOTES:
1. LOADS, LOAD COMBINATIONS AND FORCES SHALL BE PER THE ASCE 7-10. (3)

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CURRENT DATE: 05/17/19

STATE OF ILLINOIS
JACQUES L REYNOLDS
NUMBER A-2007030508
ARCHITECT

LICENSE EXPIRES: 12/31/19

CURRENT DATE: 05/17/19

STATE OF MISSOURI
TODD J. MOORE
NUMBER 2010785
REGISTERED PROFESSIONAL ENGINEER

LICENSE EXPIRES: 12/31/19

NEW BUILDING FOR:
PALMYRA UNITED METHODIST CHURCH
PALMYRA, MO 63461

CONSTRUCTION DOCUMENTS PHASE

FOR CONSTRUCTION

SET ISSUE DATE: **05/17/19**

REVISIONS

	DATE	REMARKS
1	06/07/2019	ADDENDUM 01
2	06/11/2019	ADDENDUM 02
3	06/12/2019	ADDENDUM 03

PROJECT NUMBER: **5356**

STRUCTURAL NOTES

DWG. NO.
S002